

Chattering: An overlooked peculiarity of rocking motion

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Abstract *Complete chattering* occurs when a structure undergoes a theoretically infinite sequence of impacts in finite time, that eventually bring the structure to the state of persistent (continuous) contact. This study investigates the conditions under which a rigid rocking block undergoes complete chattering when subjected to sinusoidal ground excitation. The analysis explains how the acceleration amplitude of the ground excitation affects the chattering time. It also proves that there exists a (sinusoidal) ground acceleration amplitude, below which rocking motion terminates even under a nonzero ground excitation, almost independently of the frequency of the ground excitation. Furthermore, the study adopts perturbation theory and proposes an asymptotic approximation of the time needed for chattering to be completed, i.e. *chattering time*. It then verifies the asymptotic approximation using an independent semi-analytical approach. Overall, the results highlight the importance of complete chattering on the dynamic rocking response; a feature of nonlinear dynamics which is often overlooked in earthquake engineering.

Keywords rocking · chattering · perturbation theory · asymptotic analysis · impact

1 Introduction

Rocking motion is increasingly being investigated as a means to isolate structures from ground excitations by allowing rigid body rotation. Subsequently, it finds application on a variety of engineering projects, e.g. from freestanding contents [22, 39, 40] and classical monuments [23, 49, 63] to modern buildings and bridges [2, 18, 21, 26, 28, 54, 59–61]. From an earthquake engineering perspective, the transient response of a rocking structure under a finite duration ground excitation is of primary importance, as it directly relates to seismic safety and structural failure [15, 19, 45, 55, 56, 62]. The dynamics of a (slender) freestanding rigid structure rocking on a rigid surface, without sliding or bouncing, when subjected to horizontal ground excitation is a classical problem which has been investigated in various scientific fields [35]. Despite the apparent structural simplicity, rocking motion is characterised by various nonlinear and nonsmooth dynamic phenomena, which compose a complex [64] and often chaotic behaviour [33, 37, 46].

Chattering is a feature of nonlinear dynamics that might appear during low amplitude oscillations, and is also evident in rocking motion. Chattering can be either *complete* or *incomplete* [8, 51]. Complete chattering refers to a sequence of theoretically infinite in number low velocity impacts that occur in finite time, and result in the structure coming to rest, even under nonzero ground excitation. Chattering is completed when a change in the state of contact, from instantaneous (i.e. impacts) to persistent (i.e. continuous con-

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tact), takes place. At that time-instant, an *accumulation* (or *Zeno*) point appears in the response. Incomplete chattering refers to the case in which the sequence of low velocity impacts terminates without bringing the structure to rest. An intuitive illustration of complete chattering is a ball bouncing under gravity on an obstacle, exhibiting a theoretically infinite number of impacts till it comes to rest in finite time [14, 32, 43, 47, 53, 57]. Complete chattering is also observed in the motion of a falling rod [30, 31], the motion of the Euler’s disk just before reaching its final singularity [42], and the inverted pendulum bouncing on lateral side-walls [8, 13, 44, 58].

Chattering is a highly nonlinear phenomenon that has been investigated in many fields of engineering (see e.g. Brogliato [6] and references therein). Two main research questions concern (i) the effect of chattering on the dynamic response, and (ii) the numerical challenges that emerge in simulations due to the presence of accumulation points.

With reference to the first question, Budd and Dux [8] showed that chattering is one of the main reasons of chaotic behaviour of impact oscillators. That study also revealed that small changes at the initial conditions can transform chattering from complete to incomplete. Baranyai and Várkonyi [5] investigated the initial conditions that engender complete chattering, and subsequently accumulation points, assuming frictionless impacts and ignoring any external force. A series of studies proposed Lyapunov-like conditions sufficient to restore stability to the structural system (i.e. Zeno stability) [29, 41]. Fewer studies focused on the time required till the accumulation point emerges, i.e. *chattering time*. Leine and Heimesch [43] and Or and Ames [52] adopted the Lyapunov stability theory [48] and proposed an upper bound of the finite chattering time for the bouncing ball problem. To address the same question, Demeio and Lenci [13] examined the inverted pendulum impacting on lateral side-walls and approximated the chattering time using asymptotic analysis.

The appearance of accumulation points creates numerical challenges/instabilities during simulation [52] as the persistent contact (corresponding to the accumulation point) is not described by the differential equation of motion [9, 43]. Such numerical challenges become even more complicated when impacts occur at multiple contact points. Different methods have been proposed to tackle such instability issues caused by the infinite number of impacts in finite time. One way is to bypass accumulation points (Nordmark and Piiroinen [51]) using an event-based algorithm, and thus, considering only finite, in number, impacts. Another approach is to approximate the accumulation points

(Ames et al. [3]). It is also possible to suppress the spurious oscillations due to chattering (Acary [1]) via modifying the Moreau–Jean’s time-stepping scheme [36, 50], increasing somewhat the energy dissipation. A popular approach is to adopt a velocity threshold to artificially terminate the motion of the structure once the velocity drops below that threshold (Chatterjee et al. [9] and Cosimo et al. [10]). A velocity threshold approach to tackle chattering in rocking simulations was also adopted in [18, 26, 27]. Alternatively, Cusumano and Bai [11] and Wagg and Bishop [65] applied a threshold on the time-interval between consecutive impacts to terminate chattering motion.

The motivation for this study is to bring forward (complete) chattering and its significance on rocking response, which has been rarely considered in rocking dynamics and earthquake engineering. Focusing on sinusoidal excitation, the specific aim is to determine the conditions under which chattering occurs, approximate how long it takes for chattering to complete (chattering time), and offer a glimpse into its implications on rocking dynamics. For this purpose, this study (i) adopts perturbation theory [34] and proposes, for the first time, an asymptotic approximation of chattering time for the rocking problem, (ii) demonstrates that within this (chattering) time-frame, the rocking block can come to rest even though the ground excitation is still active, and (iii) discusses implications of chattering using numerical simulations.

2 Dynamics of the rocking block

The rigid rocking block is an archetypal structural model that describes a wide class of rocking configurations [12], and a typical example of nonsmooth dynamics, in which smooth motion is interrupted by nonsmooth velocity “jumps”, i.e. impacts [7, 25]. The present study focuses on the transient response of a rocking block when subjected to multi-lobe mathematical ground excitations of low amplitude and finite duration.

2.1 Rocking initiation

Consider the rigid block of Fig. 1 under a horizontal ground acceleration time-history \ddot{u}_g . Assuming the block does not slide at the contact interface and it is slender enough to avoid bouncing effects, rocking commences once the seismic demand, i.e. the moment of the inertia forces with respect to the pivot point O (or O’), exceeds the seismic resistance, i.e. the moment of the weight with respect to the same pivot point. This moment equilibrium condition yields the minimum ground

acceleration $a_{g,\min}$ necessary to initiate rocking.

$$\ddot{u}_g(t) \geq a_{g,\min} = g \tan \alpha \quad (1)$$

where g denotes the gravitational acceleration, while α is the slenderness angle ($\alpha = \tan^{-1}(b/h)$) with b being the block's half width and h being the half height (Fig. 1). For slender structures, Eq. (1) becomes: $a_{g,\min} \approx g\alpha$.

2.2 Equations of motion and closed-form solutions

After rocking initiates, the continuous smooth motion of the block is interrupted by nonsmooth impacts at the pre-defined pivot (contact) points (Fig. 1). Moment equilibrium with respect to the pivot point O (or O') yields the equations which describe the continuous smooth rocking motion of the block:

$$\begin{aligned} I_0 \ddot{\theta} + mgR \sin(\alpha - \theta) &= -m\ddot{u}_g R \cos(\alpha - \theta), & \theta > 0 \\ I_0 \ddot{\theta} + mgR \sin(-\alpha - \theta) &= -m\ddot{u}_g R \cos(-\alpha - \theta), & \theta < 0 \end{aligned} \quad (2)$$

where I_0 represents the moment of inertia of the rigid body with respect to the pivot point O (or O'), m is the mass of the block, R is the half-diagonal distance measured from the pivot point to the centre of mass (C.M.), and θ is the rocking (rigid body) rotation (Fig. 1). Equation (2) is a set of ordinary differential equations that depends on the sign of θ . For slender structures, Eq. (2) can be linearised to [17]:

$$\frac{\ddot{\theta}}{p^2 \alpha} - \frac{\theta}{\alpha} + \text{sgn}(\theta) = -\frac{\ddot{u}_g}{g\alpha} \quad (3)$$

$p = \sqrt{3g/(4R)}$ in Eq. (3) denotes the frequency parameter of the block, while $\text{sgn}(\theta)$ is the standard sign function (i.e. $\text{sgn}(\theta) = 1$ when $\theta > 0$, $\text{sgn}(\theta) = 0$ when $\theta = 0$, and $\text{sgn}(\theta) = -1$ when $\theta < 0$). The dynamic behaviour of the rigid rocking block of Fig. 1 is described by the dimensionless equation of Eq. (3), which for a (harmonic) sinusoidal excitation can be written as [17]:

$$\phi_n''(\tau) - \phi_n(\tau) + \text{sgn}(\phi_n) = -a \sin(\omega\tau + \psi), \quad \tau \leq \tau_{ex} \quad (4)$$

where τ is the dimensionless time and τ_{ex} is the dimensionless time-instant at which the ground excitation ends, while ψ is the phase angle when rocking commences. $\phi(\tau)$ is the dimensionless rotation at time τ , and $\phi''(\tau)$ is the dimensionless angular acceleration at time τ . a is the dimensionless amplitude of the horizontal ground acceleration, and ω is the frequency

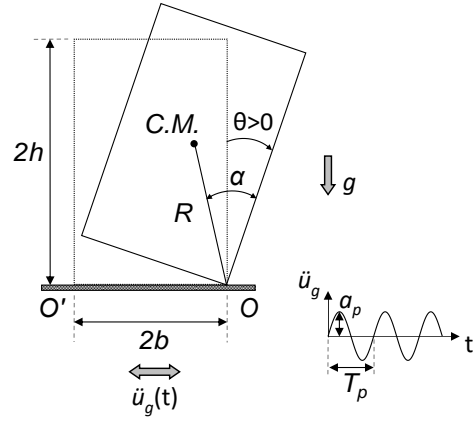


Fig. 1 The archetypal rigid rocking block under horizontal ground excitation.

ratio with ω_g being the frequency of the ground excitation:

$$\begin{aligned} \phi_n'' &= \frac{\ddot{\theta}_n}{p^2 \alpha}, & \phi_n &= \frac{\theta_n}{\alpha}, & \tau &= pt, \\ \omega &= \frac{\omega_g}{p}, & a &= \frac{\ddot{u}_g}{g\alpha} \end{aligned} \quad (5)$$

Subscript n refers to the number of impacts that have already occurred during rocking motion.

Conveniently, Eq. (4) allows for an analytical solution, which can be expressed as [17]:

$$\begin{aligned} \phi_n(\tau) &= B_n e^\tau + A_n e^{-\tau} + i_n + \Xi(\tau) \\ \phi_n'(\tau) &= B_n e^\tau - A_n e^{-\tau} + \Xi'(\tau) \end{aligned} \quad (6)$$

with derivatives:

$$\begin{aligned} \phi_n''(\tau) &= B_n e^\tau + A_n e^{-\tau} + \Xi''(\tau) \\ \phi_n^{(3)}(\tau) &= B_n e^\tau - A_n e^{-\tau} + \Xi^{(3)}(\tau) \\ \phi_n^{(4)}(\tau) &= B_n e^\tau + A_n e^{-\tau} + \Xi^{(4)}(\tau) \\ \phi_n^{(5)}(\tau) &= B_n e^\tau - A_n e^{-\tau} + \Xi^{(5)}(\tau) \end{aligned} \quad (7)$$

where A_n, B_n are terms that depend on the initial conditions. i_n represents the sign of rocking rotation and is equal to $i_n = \pm 1$. Specifically, when the rocking block undergoes clockwise rotation $i_n = 1$, otherwise $i_n = -1$. Assuming the ground acceleration is initially positive, the initial rotation of the block is counter-clockwise, hence, without loss of generality, for $n = 0$ it follows $i_0 = -1$. The term $\Xi(\tau)$ represents the particular solution of the differential equation of Eq. (4), which captures the effect of the specific ground motion on rocking response. For a sinusoidal ground acceleration, $\Xi(\tau)$ takes the form [16]:

$$\begin{aligned} \Xi(\tau) &= \frac{1}{\omega^2 + 1} a \sin(\omega\tau + \psi) \\ \Xi'(\tau) &= \frac{\omega}{\omega^2 + 1} a \cos(\omega\tau + \psi) \end{aligned} \quad (8)$$

with derivatives:

$$\begin{aligned}\Xi''(\tau) &= -\frac{\omega^2}{\omega^2+1}a \sin(\omega\tau + \psi) \\ \Xi^{(3)}(\tau) &= -\frac{\omega^3}{\omega^2+1}a \cos(\omega\tau + \psi) \\ \Xi^{(4)}(\tau) &= \frac{\omega^4}{\omega^2+1}a \sin(\omega\tau + \psi) \\ \Xi^{(5)}(\tau) &= \frac{\omega^5}{\omega^2+1}a \cos(\omega\tau + \psi)\end{aligned}\quad (9)$$

For $n = 0$ and $\tau = \tau_0 = 0$, i.e. the time-instant rocking commences (Fig. 2(b)), the initial conditions of the rocking block of Fig. 1 are: $\phi_0(\tau_0) = 0$ and $\phi'_0(\tau_0) = 0$. Hence, Eq. (6) gives [16]:

$$\begin{aligned}A_0 &= -\frac{1}{2} \left[\Xi(\tau_0) - \Xi'(\tau_0) + i_0 \right] \\ B_0 &= -\frac{1}{2} \left[\Xi(\tau_0) + \Xi'(\tau_0) + i_0 \right]\end{aligned}\quad (10)$$

where from Eq. (8) for $\tau = \tau_0 = 0$:

$$\begin{aligned}\Xi(\tau_0) &= \frac{1}{\omega^2+1}a \sin(\psi) \\ \Xi'(\tau_0) &= \frac{\omega}{\omega^2+1}a \cos(\psi)\end{aligned}\quad (11)$$

In addition, the initial conditions of $\phi_0(\tau_0) = 0$ and $\phi''_0(\tau_0) = 0$ using Eq. (4) give the phase angle when rocking initiates:

$$\psi = \sin^{-1} \left(\frac{-i_0}{a} \right)\quad (12)$$

2.3 Treatment of impact

Impact occurs whenever the rocking rotation switches sign ($\phi_n(\tau_n) = 0$). Assuming pure rocking behaviour (i.e. no sliding and bouncing effects during rocking), the block, after impact, switches pivot point and continues its smooth rocking motion about the new pivot point. Due to the impulsive nature of impact, at $\tau = \tau_n$ when impact happens, an instantaneous decrease in velocity occurs (i.e., nonsmooth velocity “jump” [7, 25]). Hence, the angular velocities before ($\phi'_{n-1}(\tau_n)$) and after ($\phi'_n(\tau_n)$) impact are connected through the (dimensionless) coefficient of restitution η [35].

$$\phi'_n(\tau_n) = \eta \phi'_{n-1}(\tau_n)\quad (13)$$

One way to analytically estimate the coefficient of restitution η is via the conservation of moment of momentum, just before and just after impact [35]. Yet, this is merely a theoretical approximation, since the amount of (kinetic) energy lost at impact is not solely a function

of the block’s geometry (see e.g. [4, 20, 38] and references therein). Thus, the value of coefficient of restitution η is usually inverse fitted from experiments [20] or assumed [27]. This study considers the coefficient of restitution an independent parameter able to incorporate any of the above modifications.

3 Asymptotic approximation of the chattering time

This section investigates the chattering behaviour of a rocking block under a low amplitude (i.e. weak) ground excitation. It specifically aims to approximate asymptotically the duration of chattering motion τ_{ch} , namely the finite time the block takes to come to rest through a theoretically infinite sequence of impacts under weak ground excitation. The proposed asymptotic approach builds on the pioneering study of the inverted pendulum problem by Demeio and Lenci [13]. The rocking problem examined herein differs from the inverted pendulum as after every impact the equation of motion changes (as reflected in Eq. (6) by the value of i_n), which further complicates the proposed approach.

The premise of this asymptotic approach is that the chattering time is the total duration the sequence of impacts last till the block comes to rest. This duration equals the sum of the time-intervals $\Delta\tau$ between these infinite consecutive impacts (Fig. 2). The time-instant $\tau = \tau_{n+1}$ the $(n+1)^{th}$ impact occurs is determined from the expression of rocking rotation initiated after the n^{th} impact $\phi_n(\tau_{n+1}) = 0$. To calculate τ_{n+1} , one must first calculate the initial conditions of the cycle of motion after the n^{th} impact. This necessitates the derivation of general expressions of the angular velocity $\phi'_n(\tau_n)$ of the block at (any) impact n , and of the time-interval $\Delta\tau_n$ between consecutive impacts at τ_n and τ_{n+1} . Accordingly, the approach treats separately the first cycle of rocking motion (Section 3.1) from all subsequent cycles of rocking motion (Sections 3.2, 3.3), as their initial conditions (and specifically the angular velocity) differ.

For the sake of analytical solutions, this section considers a (harmonic) sinusoidal ground excitation and adopts perturbation theory [34] to expand the rocking rotation $\phi_n(\tau)$ and the time-interval $\Delta\tau_n$ in power series of a small parameter ε . The parameter ε relates to the dimensionless amplitude of the ground acceleration a :

$$a = \frac{\ddot{u}_g}{g\alpha} = \varepsilon + 1\quad (14)$$

Note that $a = 1$ corresponds to $\varepsilon = 0$ and represents the critical acceleration value above which ($a \geq 1$)

rocking commences (Eq. (1)). Specifically, a small positive ε value indicates a weak ground excitation, barely capable of triggering rocking motion. Thus, the rocking problem examined herein is governed by the dimensionless acceleration amplitude a and frequency ω of the ground excitation (Eq. (5)) and the dimensionless coefficient of restitution η (Eq. (13)) that controls the energy dissipation at impact.

In this context, the rocking rotation can be written as [13]:

$$\phi_n(\tau) = \sum_{m=0}^M \Phi_n^{(m)}(\tau - \tau_n)^m \quad (15)$$

where

$$\Phi_n^{(m)} = \frac{1}{m!} \frac{d^m \phi_n}{d\tau^m}(\tau_n) \quad (16)$$

m defines the order of time-derivative, while n refers to the number of impacts. Equation (15), when the $(n+1)^{th}$ impact happens (i.e. at $\tau = \tau_{n+1}$), gives:

$$\sum_{m=0}^M \Phi_n^{(m)}(\Delta\tau_n)^m = 0 \quad (17)$$

where $\Delta\tau_n = \tau_{n+1} - \tau_n$ denotes the time-interval between two consecutive impacts that occur at the time-instants τ_n and τ_{n+1} (Fig. 2(c)). Using the definition of $\Phi_n^{(m)}$ (Eq. (16)), Eq. (17) reads:

$$\phi_n(\tau_n) + \sum_{m=1}^M \Phi_n^{(m)}(\Delta\tau_n)^m = 0 \quad (18)$$

where $\phi_n(\tau_n) = 0$ since impact happens. Hence, Eq. (18) becomes:

$$\Delta\tau_n \sum_{m=1}^M \Phi_n^{(m)}(\Delta\tau_n)^{m-1} = 0 \quad (19)$$

which implies that at the time-instant τ_{n+1} the $(n+1)^{th}$ impact happens ($\phi_n(\tau_{n+1}) = 0$):

$$\sum_{m=0}^{M-1} \Phi_n^{(m+1)}(\Delta\tau_n)^m = 0 \quad (20)$$

3.1 First cycle of rocking motion

This section approximates the angular velocity of the rocking block $\phi_0'(\tau_1)$ at the time-instant of the first impact $\tau = \tau_1$. To this end, it first approximates τ_1 , or equivalently $\Delta\tau_0 = \tau_1 - \tau_0$, from Eq. (20) for $n = 0$, namely $\phi_0(\tau_1) = 0$.

During the first cycle of rotation (i.e. $n = 0$ in Fig. 2(b)), Eq. (15) can be expanded in Taylor series centred at the time-instant $\tau_0 = 0$ rocking initiates:

$$\begin{aligned} \phi_0(\tau) &= \sum_{m=0}^M \Phi_0^{(m)}(\tau - \tau_0)^m \\ &= \sum_{m=0}^M \left[\frac{1}{m!} \frac{d^m \phi_0}{d\tau^m}(\tau_0) \right] (\tau - \tau_0)^m \end{aligned} \quad (21)$$

Substituting Eqs (6), (7) with the aid of Eq. (10) into Eq. (21) and re-ordering the terms:

$$\begin{aligned} \phi_0(\tau) &= \frac{1}{3!} \left[\Xi^{(3)}(\tau_0) - \Xi'(\tau_0) \right] (\tau - \tau_0)^3 \\ &+ \frac{1}{4!} \left[\Xi^{(4)}(\tau_0) - \Xi''(\tau_0) \right] (\tau - \tau_0)^4 \\ &+ \frac{1}{5!} \left[\Xi^{(5)}(\tau_0) - \Xi'(\tau_0) \right] (\tau - \tau_0)^5 + \dots \end{aligned} \quad (22)$$

Equations (21), (22) give:

$$\begin{aligned} \Phi_0^{(0)} &= 0 \\ \Phi_0^{(1)} &= 0 \\ \Phi_0^{(2)} &= 0 \\ \Phi_0^{(3)} &= \frac{1}{3!} \left[\Xi^{(3)}(\tau_0) - \Xi'(\tau_0) \right] \\ \Phi_0^{(4)} &= \frac{1}{4!} \left[\Xi^{(4)}(\tau_0) - \Xi''(\tau_0) \right] \\ \Phi_0^{(5)} &= \frac{1}{5!} \left[\Xi^{(5)}(\tau_0) - \Xi'(\tau_0) \right] \end{aligned} \quad (23)$$

Equation (23) implies that, at $\tau_0 = 0$, the rotation $\Phi_0^{(0)}$, the angular velocity $\Phi_0^{(1)}$, and the angular acceleration $\Phi_0^{(2)}$ are zero. The angular jerk $\Phi_0^{(3)}$ is nonzero as it describes the rate of change of the angular acceleration at the time-instant rocking initiates. The $\Xi^{(m)}(\tau_0)$ terms in Eq. (23) are related to the forcing function (Eqs (8), (9)). Because of Eq. (14), the trigonometric terms of Eqs (8), (9), (11) become:

$$\begin{aligned} a \sin(\omega\tau_0 + \psi) &= a \sin(\psi) = -i_0 \\ a \cos(\omega\tau_0 + \psi) &= a \cos(\psi) = \sqrt{2\varepsilon + \varepsilon^2} \end{aligned} \quad (24)$$

The second equation of Eq. (24) can be expanded in Taylor series in powers of ε :

$$\begin{aligned} a \cos(\omega\tau_0 + \psi) &= a \cos(\psi) \\ &= \sqrt{2} \left[\varepsilon^{\frac{1}{2}} + \frac{\varepsilon^{\frac{3}{2}}}{4} - \frac{\varepsilon^{\frac{5}{2}}}{32} + O\left(\varepsilon^{\frac{7}{2}}\right) \right] \end{aligned} \quad (25)$$

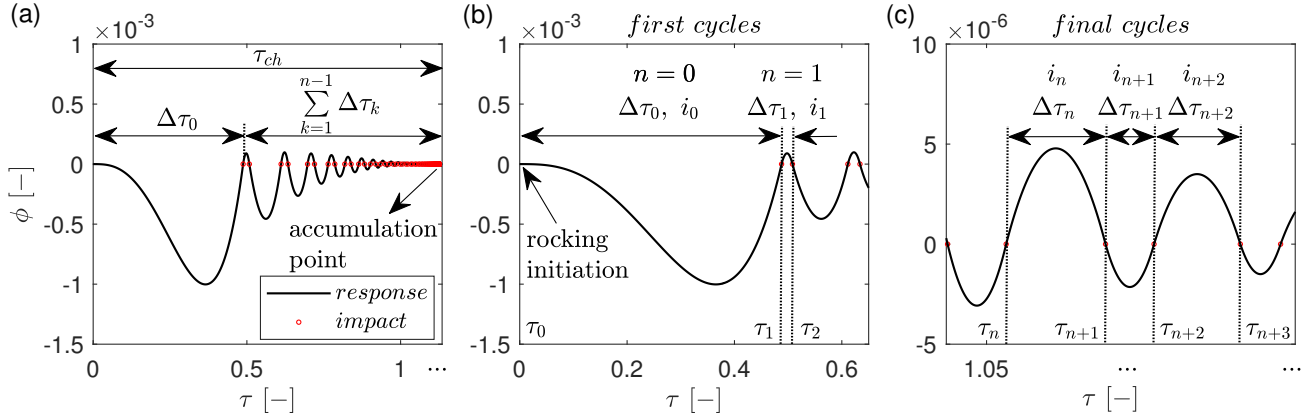


Fig. 2 (a) Chattering oscillations of the rocking block ($p = 1 \text{ s}^{-1}$ and $\alpha = 0.2 \text{ rad}$) of Fig. 1 when subjected to a sinusoidal ground excitation of amplitude $a = 1.03$ and frequency $\omega = 2$ for $\eta = 0.92$, with details of the (b) first and (c) final response cycles.

Therefore, Eqs (8), (9) through Eqs (24), (25) for $\tau = \tau_0$ become:

$$\begin{aligned}
 \Xi(\tau_0) &= -\frac{i_0}{\omega^2 + 1} \\
 \Xi'(\tau_0) &= \frac{\omega}{\omega^2 + 1} \sqrt{2} \left[\varepsilon^{\frac{1}{2}} + \frac{\varepsilon^{\frac{3}{2}}}{4} - \frac{\varepsilon^{\frac{5}{2}}}{32} + O\left(\varepsilon^{\frac{7}{2}}\right) \right] \\
 \Xi''(\tau_0) &= \frac{\omega^2 i_0}{\omega^2 + 1} \\
 \Xi^{(3)}(\tau_0) &= -\frac{\omega^3}{\omega^2 + 1} \sqrt{2} \left[\varepsilon^{\frac{1}{2}} + \frac{\varepsilon^{\frac{3}{2}}}{4} - \frac{\varepsilon^{\frac{5}{2}}}{32} + O\left(\varepsilon^{\frac{7}{2}}\right) \right] \\
 \Xi^{(4)}(\tau_0) &= -\frac{\omega^4 i_0}{\omega^2 + 1} \\
 \Xi^{(5)}(\tau_0) &= \frac{\omega^5}{\omega^2 + 1} \sqrt{2} \left[\varepsilon^{\frac{1}{2}} + \frac{\varepsilon^{\frac{3}{2}}}{4} - \frac{\varepsilon^{\frac{5}{2}}}{32} + O\left(\varepsilon^{\frac{7}{2}}\right) \right]
 \end{aligned} \tag{26}$$

Subsequently, Eq. (23) through Eq. (26) reads:

$$\begin{aligned}
 \Phi_0^{(0)} &= 0 \\
 \Phi_0^{(1)} &= 0 \\
 \Phi_0^{(2)} &= 0 \\
 \Phi_0^{(3)} &= -\frac{\omega}{6} \sqrt{2} \left[\varepsilon^{\frac{1}{2}} + \frac{\varepsilon^{\frac{3}{2}}}{4} - \frac{\varepsilon^{\frac{5}{2}}}{32} + O\left(\varepsilon^{\frac{7}{2}}\right) \right] \\
 \Phi_0^{(4)} &= -\frac{\omega^2 i_0}{24} \\
 \Phi_0^{(5)} &= \frac{\omega(\omega^2 - 1)}{120} \sqrt{2} \left[\varepsilon^{\frac{1}{2}} + \frac{\varepsilon^{\frac{3}{2}}}{4} - \frac{\varepsilon^{\frac{5}{2}}}{32} + O\left(\varepsilon^{\frac{7}{2}}\right) \right]
 \end{aligned} \tag{27}$$

Observe that each order of time-derivative in Eq. (27) can be expressed in the general form:

$$\begin{aligned}
 \Phi_0^{(m)} &= \Phi_{0;0}^{(m)} + \Phi_{0;1}^{(m)} \varepsilon^{\frac{1}{2}} + \Phi_{0;3}^{(m)} \varepsilon^{\frac{3}{2}} + \Phi_{0;5}^{(m)} \varepsilon^{\frac{5}{2}} + O\left(\varepsilon^{\frac{7}{2}}\right) \\
 &= \Phi_{0;0}^{(m)} + \sum_{l=0}^L \Phi_{0;2l+1}^{(m)} \varepsilon^{\frac{2l+1}{2}}
 \end{aligned} \tag{28}$$

where, the superscript m of e.g. $\Phi_{0;1}^{(m)}$ indicates the order of differentiation, and the subscript “0;1” of $\Phi_{0;1}^{(m)}$ represents the number of impacts with the first subscript “0”, and the order of approximation l in powers of ε with the second subscript “1”. The expansion of $\Phi_0^{(m)}$ in half-integer powers of ε (Eqs (27), (28)) suggests an expansion of $\Delta\tau_0$ (Fig. 2(b)) also in half-integer powers of ε :

$$\begin{aligned}
 \Delta\tau_0 &= \tau_1 - \tau_0 \\
 &= T_{0;0} + T_{0;1} \varepsilon^{\frac{1}{2}} + T_{0;2} \varepsilon^{\frac{3}{2}} + T_{0;3} \varepsilon^{\frac{5}{2}} + O\left(\varepsilon^{\frac{4}{2}}\right)
 \end{aligned} \tag{29}$$

Expanding Eq. (20) for $n = 0$ and using Eqs (28), (29):

$$\begin{aligned}
 &\Phi_{0;0}^{(4)} T_{0;0} + \left[\Phi_{0;1}^{(3)} + \Phi_{0;0}^{(4)} T_{0;1} + \Phi_{0;1}^{(5)} (T_{0;0})^2 \right] \varepsilon^{\frac{1}{2}} \\
 &+ \left[\Phi_{0;0}^{(4)} T_{0;2} + 2\Phi_{0;1}^{(5)} T_{0;0} T_{0;1} \right] \varepsilon^{\frac{3}{2}} \\
 &+ \left[\Phi_{0;3}^{(3)} + \Phi_{0;0}^{(4)} T_{0;3} + \Phi_{0;1}^{(5)} T_{0;1}^2 \right. \\
 &\left. + 2\Phi_{0;1}^{(5)} T_{0;0} T_{0;2} + \Phi_{0;3}^{(5)} T_{0;0}^2 \right] \varepsilon^{\frac{5}{2}} + O\left(\varepsilon^{\frac{4}{2}}\right) = 0
 \end{aligned} \tag{30}$$

Equation (30) yields a hierarchy of equations in powers of ε , the solution of which returns the terms of Eq. (29):

$$\begin{aligned} \varepsilon^{\frac{0}{2}} : T_{0;0} &= 0 \\ \varepsilon^{\frac{1}{2}} : T_{0;1} &= -\frac{4\sqrt{2}}{\omega i_0} \\ \varepsilon^{\frac{2}{2}} : T_{0;2} &= 0 \\ \varepsilon^{\frac{3}{2}} : T_{0;3} &= \frac{\sqrt{2}}{\omega i_0} \left[-1 + \frac{32(\omega^2 - 1)}{5\omega^2} \right] \end{aligned} \quad (31)$$

Hence, the time-instant the first impact happens becomes (Eq. (29)):

$$\tau_1 = -\frac{4\sqrt{2}}{\omega i_0} \varepsilon^{\frac{1}{2}} + \frac{\sqrt{2}}{\omega i_0} \left[-1 + \frac{32(\omega^2 - 1)}{5\omega^2} \right] \varepsilon^{\frac{3}{2}} + O\left(\varepsilon^{\frac{4}{2}}\right) \quad (32)$$

Differentiation of Eq. (15) for $\tau = \tau_1$ and $n = 0$ gives the angular velocity at first impact (τ_1):

$$\phi'_0(\tau_1) = \sum_{m=0}^{M-1} (m+1) \Phi_0^{(m+1)}(\Delta\tau_0)^m \quad (33)$$

Substituting Eqs (27), (29) into the expanded form of Eq. (33) returns the angular velocity at first impact (τ_1):

$$\phi'_0(\tau_1) = \frac{16\sqrt{2}}{3\omega} \varepsilon^{\frac{3}{2}} + \frac{4\sqrt{2}}{\omega} \left[1 - \frac{32(\omega^2 - 1)}{15\omega^2} \right] \varepsilon^{\frac{5}{2}} + O\left(\varepsilon^{\frac{7}{2}}\right) \quad (34)$$

from which, based on Eq. (28):

$$\begin{aligned} \varepsilon^{\frac{0}{2}} : \Phi_{0;0}^{(1)} &= 0 \\ \varepsilon^{\frac{1}{2}} : \Phi_{0;1}^{(1)} &= 0 \\ \varepsilon^{\frac{3}{2}} : \Phi_{0;3}^{(1)} &= \frac{16\sqrt{2}}{3\omega} \\ \varepsilon^{\frac{5}{2}} : \Phi_{0;5}^{(1)} &= \frac{4\sqrt{2}}{\omega} \left[1 - \frac{32(\omega^2 - 1)}{15\omega^2} \right] \end{aligned} \quad (35)$$

Therefore, the angular velocity of the rocking block after the first impact is given from the angular velocity before impact (Eq. (34)) with the aid of the coefficient of restitution η (Eq. (13)): $\phi'_1(\tau_1) = \eta\phi'_0(\tau_1)$.

3.2 Subsequent cycles of rocking motion

For all subsequent impacts, the calculation steps are similar as in Section 3.1 with the main difference being the initial conditions (and specifically the angular velocity) after each impact. In particular, after the first

impact (i.e. $n = 1$ in Fig. 2(b)) at $\tau = \tau_1$, the new initial conditions $\phi_1(\tau_1) = 0$ and $\phi'_1(\tau_1) \neq 0$ according to Eq. (6) give:

$$\begin{aligned} B_1 e^{\tau_1} + A_1 e^{-\tau_1} &= -i_1 - \Xi(\tau_1) \\ B_1 e^{\tau_1} - A_1 e^{-\tau_1} &= \phi'_1(\tau_1) - \Xi'(\tau_1) \end{aligned} \quad (36)$$

in which $i_1 = -i_0$, as the sign of rotation after the first impact changes (Fig. 2(b)). Equation (15) for $n = 1$ becomes:

$$\begin{aligned} \phi_1(\tau) &= \sum_{m=0}^M \Phi_1^{(m)}(\tau - \tau_1)^m \\ &= \sum_{m=0}^M \left[\frac{1}{m!} \frac{d^m \phi_1}{d\tau^m}(\tau_1) \right] (\tau - \tau_1)^m \end{aligned} \quad (37)$$

Expansion of Eq. (37) considering Eqs (6), (7), (36) yields:

$$\begin{aligned} \phi_1(\tau) &= \frac{1}{1!} \phi'_1(\tau_1) (\tau - \tau_1) \\ &+ \frac{1}{2!} \left[-i_1 - \Xi(\tau_1) + \Xi''(\tau_1) \right] (\tau - \tau_1)^2 \\ &+ \frac{1}{3!} \left[\phi'_1(\tau_1) - \Xi'(\tau_1) + \Xi^{(3)}(\tau_1) \right] (\tau - \tau_1)^3 \\ &+ \frac{1}{4!} \left[-i_1 - \Xi(\tau_1) + \Xi^{(4)}(\tau_1) \right] (\tau - \tau_1)^4 + \dots \end{aligned} \quad (38)$$

Equations (37), (38) give:

$$\begin{aligned} \Phi_1^{(0)} &= 0 \\ \Phi_1^{(1)} &= \phi'_1(\tau_1) \\ \Phi_1^{(2)} &= \frac{1}{2!} \left[-i_1 - \Xi(\tau_1) + \Xi''(\tau_1) \right] \\ \Phi_1^{(3)} &= \frac{1}{3!} \left[\phi'_1(\tau_1) - \Xi'(\tau_1) + \Xi^{(3)}(\tau_1) \right] \\ \Phi_1^{(4)} &= \frac{1}{4!} \left[-i_1 - \Xi(\tau_1) + \Xi^{(4)}(\tau_1) \right] \end{aligned} \quad (39)$$

Alternatively, Eq. (39) for $n = 1$ can be expressed as:

$$\begin{aligned} \Phi_1^{(2m+1)} &= \frac{1}{(2m+1)!} \left[\phi'_1(\tau_1) - \Xi^{(1)}(\tau_1) + \Xi^{(2m+1)}(\tau_1) \right] \\ \Phi_1^{(2m)} &= \frac{1}{(2m)!} \left[-i_1 - \Xi(\tau_1) + \Xi^{(2m)}(\tau_1) \right] \end{aligned} \quad (40)$$

Thus, in general, for any impact n , Eq. (40) becomes:

$$\begin{aligned} \Phi_n^{(2m+1)} &= \frac{1}{(2m+1)!} \left[\phi'_n(\tau_n) - \Xi^{(1)}(\tau_n) \right], \quad m \geq 0 \\ \Phi_n^{(2m)} &= \frac{1}{(2m)!} \left[-i_n - \Xi(\tau_n) + \Xi^{(2m)}(\tau_n) \right], \quad m > 0 \end{aligned} \quad (41)$$

3.3 Chattering time

As the rocking block continues its chattering oscillations subjected to ground excitation, its velocity lessens at every impact till it eventually becomes zero after a theoretically infinite number of impacts (Fig. 2(a)). This section approximates the time needed for the block to come to rest, i.e. chattering time, while subjected to a nonzero (weak) ground excitation.

The chattering time is equal with the total duration of the sequence of impacts (Fig. 2(a)):

$$\tau_{ch} = \sum_{n=0}^{\infty} \Delta\tau_n \quad (42)$$

where n represents the number of impacts. Following Eq. (29), $\Delta\tau_n$ can be expanded as:

$$\begin{aligned} \Delta\tau_n &= \tau_{n+1} - \tau_n \\ &= T_{n;0} + T_{n;1}\varepsilon^{\frac{1}{2}} + T_{n;2}\varepsilon^{\frac{3}{2}} + T_{n;3}\varepsilon^{\frac{5}{2}} + O\left(\varepsilon^{\frac{7}{2}}\right) \\ &= \sum_{l=0}^L T_{n;l}\varepsilon^{\frac{l}{2}} \end{aligned} \quad (43)$$

where, again, l refers to the order of approximation. Thus, at any time-instant of impact $\phi_n(\tau_n) = 0$, Eq. (20) gives:

$$\Phi_n^{(1)} + \Phi_n^{(2)} \Delta\tau_n + \Phi_n^{(3)} (\Delta\tau_n)^2 + \Phi_n^{(4)} (\Delta\tau_n)^3 + \dots = 0 \quad (44)$$

Recall that the term $\Phi_n^{(m)}$ in Eq. (44) represents the m^{th} derivative of rocking rotation after the n^{th} impact and is given by Eq. (41) as:

$$\begin{aligned} \Phi_n^{(1)} &= \phi_n'(\tau_n) \\ \Phi_n^{(2)} &= \frac{1}{2} \left[-i_n - \Xi(\tau_n) + \Xi^{(2)}(\tau_n) \right] \\ \Phi_n^{(3)} &= \frac{1}{6} \left[\phi_n'(\tau_n) - \Xi^{(1)}(\tau_n) + \Xi^{(3)}(\tau_n) \right] \\ \Phi_n^{(4)} &= \frac{1}{24} \left[-i_n - \Xi(\tau_n) + \Xi^{(4)}(\tau_n) \right] \end{aligned} \quad (45)$$

The $\Phi_n^{(m)}$ terms in Eq. (45) rely on the general form of the forcing function $\Xi^{(m)}(\tau_n)$ (Eqs (8), (9)):

$$\begin{aligned} \Xi^{(2m)}(\tau_n) &= \frac{\omega^{2m}(-1)^m}{\omega^2 + 1} a \sin(\omega\tau_n + \psi) \\ \Xi^{(2m+1)}(\tau_n) &= \frac{\omega^{2m+1}(-1)^m}{\omega^2 + 1} a \cos(\omega\tau_n + \psi) \end{aligned} \quad (46)$$

and the time-instant the n^{th} impact occurs τ_n :

$$\begin{aligned} \tau_n &= \tau_0 + \sum_{k=0}^{n-1} \Delta\tau_k = \tau_0 + \sum_{l=1}^L \left(\sum_{k=0}^{n-1} T_{k;l} \right) \varepsilon^{\frac{l}{2}} \\ &= \tau_0 + \sum_{l=1}^L \sigma_{n;l} \varepsilon^{\frac{l}{2}} \end{aligned} \quad (47)$$

where $\sigma_{n;l}$ represents the summation of the l^{th} order of approximation $T_{k;l}$ of all the the time-intervals $\Delta\tau_k$ between consecutive impacts.

$$\sigma_{n;l} = \sum_{k=0}^{n-1} T_{k;l} \quad (48)$$

with $\sigma_{n;0} = 0$ since $T_{n;0} = 0$ (Eq. (31)).

Using Eqs (47), (48), the trigonometric terms $\sin(\omega\tau_n + \psi)$, $\cos(\omega\tau_n + \psi)$ become:

$$\begin{aligned} \sin(\omega\tau_n + \psi) &= \sin \left[(\omega\tau_0 + \psi) + \omega \sum_{l=1}^L \sigma_{n;l} \varepsilon^{\frac{l}{2}} \right] \\ \cos(\omega\tau_n + \psi) &= \cos \left[(\omega\tau_0 + \psi) + \omega \sum_{l=1}^L \sigma_{n;l} \varepsilon^{\frac{l}{2}} \right] \end{aligned} \quad (49)$$

Series expansion of Eq. (49) gives [13]:

$$\begin{aligned} \sin \left[(\omega\tau_0 + \psi) + \omega \sum_{l=1}^L \sigma_{n;l} \varepsilon^{\frac{l}{2}} \right] &= \sum_{l=0}^L d_{n;l} \varepsilon^{\frac{l}{2}} \\ \cos \left[(\omega\tau_0 + \psi) + \omega \sum_{l=1}^L \sigma_{n;l} \varepsilon^{\frac{l}{2}} \right] &= \sum_{l=0}^L c_{n;l} \varepsilon^{\frac{l}{2}} \end{aligned} \quad (50)$$

where:

$$\begin{aligned} d_{n;0} &= -\frac{i_0}{a} \\ d_{n;1} &= 0 \\ d_{n;2} &= \frac{\sqrt{2}}{a} \omega \sigma_{n;1} + \frac{i_0}{2a} (\omega \sigma_{n;1})^2 \\ d_{n;3} &= 0 \\ c_{n;0} &= 0 \\ c_{n;1} &= \frac{\sqrt{2}}{a} + \frac{i_0}{a} \omega \sigma_{n;1} \\ c_{n;2} &= 0 \\ c_{n;3} &= \frac{\sqrt{2}}{4a} - \frac{\sqrt{2}}{2a} (\omega \sigma_{n;1})^2 - \frac{i_0}{6a} (\omega \sigma_{n;1})^3 \end{aligned} \quad (51)$$

Recall that $\sigma_{n;1}$ represents the summation of the first order of approximation of all the time-intervals $\Delta\tau_n$ between consecutive impacts (Eq. (48)). Substituting Eqs (46), (49), (50) into Eq. (45):

$$\begin{aligned} \Phi_n^{(1)} &= \phi_n'(\tau_n) \\ \Phi_n^{(2)} &= -\frac{i_n + a d_{n;0}}{2} - \frac{a}{2} d_{n;2} \varepsilon^{\frac{3}{2}} + O\left(\varepsilon^{\frac{5}{2}}\right) \\ \Phi_n^{(3)} &= \frac{1}{6} \Phi_n^{(1)} - \frac{a\omega}{6} c_{n;1} \varepsilon^{\frac{1}{2}} - \frac{a\omega}{6} c_{n;3} \varepsilon^{\frac{3}{2}} + O\left(\varepsilon^{\frac{5}{2}}\right) \\ \Phi_n^{(4)} &= \frac{-i_n + a(\omega^2 - 1) d_{n;0}}{24} + \frac{a(\omega^2 - 1)}{24} d_{n;2} \varepsilon^{\frac{3}{2}} + O\left(\varepsilon^{\frac{5}{2}}\right) \end{aligned} \quad (52)$$

The post-impact angular velocity $\phi'_n(\tau_n)$ is connected to the pre-impact angular velocity $\phi'_{n-1}(\tau_n)$ through the coefficient of restitution η (Eq. (13)). Using the general form of Eq. (33):

$$\begin{aligned}\Phi_n^{(1)} &= \phi'_n(\tau_n) = \eta \phi'_{n-1}(\tau_n) \\ &= \eta \sum_{m=0}^{M-1} (m+1) \Phi_{n-1}^{(m+1)} (\tau_n - \tau_{n-1})^m\end{aligned}\tag{53}$$

Expanding Eq. (53) and using Eq. (52), the angular velocity at any impact n becomes:

$$\Phi_n^{(1)} = \eta \left[\begin{aligned} &\Phi_{n-1}^{(1)} + 2\Phi_{n-1;0}^{(2)} T_{n-1;1} \varepsilon^{\frac{1}{2}} + \left(2\Phi_{n-1;0}^{(2)} T_{n-1;2} + \frac{1}{2} \Phi_{n-1}^{(1)} (T_{n-1;1})^2 \right) \varepsilon^{\frac{3}{2}} \\ &+ \left(2\Phi_{n-1;2}^{(2)} T_{n-1;1} + 3\Phi_{n-1;1}^{(3)} (T_{n-1;1})^2 + 2\Phi_{n-1;0}^{(2)} T_{n-1;3} \right) \varepsilon^{\frac{5}{2}} + O\left(\varepsilon^{\frac{7}{2}}\right) \\ &+ \Phi_{n-1}^{(1)} T_{n-1;1} T_{n-1;2} + 4\Phi_{n-1;0}^{(4)} (T_{n-1;1})^3 \end{aligned} \right]\tag{54}$$

Recall that, based on Eq. (34), the lowest order of approximation of the angular velocity at the first impact (τ_1) is $\varepsilon^{\frac{3}{2}}$. However, the lowest order of approximation of the angular velocity at the subsequent impacts

($\Phi_{n-1}^{(1)}$ for $n > 1$) remains unknown. Assuming that the lowest order of approximation of the angular velocity $\Phi_{n-1}^{(1)}$ is $\varepsilon^{\frac{1}{2}}$, Eq. (54) can be re-written as:

$$\Phi_n^{(1)} = \eta \left[\begin{aligned} &\left(\Phi_{n-1;1}^{(1)} + 2\Phi_{n-1;0}^{(2)} T_{n-1;1} \right) \varepsilon^{\frac{1}{2}} + \left(\Phi_{n-1;2}^{(1)} + 2\Phi_{n-1;0}^{(2)} T_{n-1;2} \right) \varepsilon^{\frac{3}{2}} \\ &+ \left(\Phi_{n-1;3}^{(1)} + 2\Phi_{n-1;0}^{(2)} T_{n-1;3} + 2\Phi_{n-1;2}^{(2)} T_{n-1;1} \right. \\ &\left. + \frac{1}{2} \Phi_{n-1;1}^{(1)} (T_{n-1;1})^2 + 3\Phi_{n-1;1}^{(3)} (T_{n-1;1})^2 + 4\Phi_{n-1;0}^{(4)} (T_{n-1;1})^3 \right) \varepsilon^{\frac{5}{2}} + O\left(\varepsilon^{\frac{7}{2}}\right) \end{aligned} \right]\tag{55}$$

Equation (55) shows that the angular velocity $\Phi_n^{(1)}$ at any impact n takes the general form:

$$\begin{aligned}\Phi_n^{(1)} &= \Phi_{n;1}^{(1)} \varepsilon^{\frac{1}{2}} + \Phi_{n;2}^{(1)} \varepsilon^{\frac{3}{2}} + \Phi_{n;3}^{(1)} \varepsilon^{\frac{5}{2}} + O\left(\varepsilon^{\frac{7}{2}}\right) \\ &= \sum_{l=0}^L \Phi_{n;l}^{(m)} \varepsilon^{\frac{l}{2}}\end{aligned}\tag{56}$$

with:

$$\begin{aligned}\Phi_{n;1}^{(1)} &= \eta \left(\Phi_{n-1;1}^{(1)} + 2\Phi_{n-1;0}^{(2)} T_{n-1;1} \right) \\ \Phi_{n;2}^{(1)} &= \eta \left(\Phi_{n-1;2}^{(1)} + 2\Phi_{n-1;0}^{(2)} T_{n-1;2} \right) \\ \Phi_{n;3}^{(1)} &= \eta \left(\begin{aligned} &\Phi_{n-1;3}^{(1)} + 2\Phi_{n-1;0}^{(2)} T_{n-1;3} + 2\Phi_{n-1;2}^{(2)} T_{n-1;1} \\ &+ \frac{1}{2} \Phi_{n-1;1}^{(1)} (T_{n-1;1})^2 + 3\Phi_{n-1;1}^{(3)} (T_{n-1;1})^2 \\ &+ 4\Phi_{n-1;0}^{(4)} (T_{n-1;1})^3 \end{aligned} \right)\end{aligned}\tag{57}$$

where e.g. $\Phi_{n;2}^{(1)}$ represents the first derivative of rocking rotation (superscript $m = 1$) for the second order

of approximation (subscript after the semicolon $l = 2$) after the n^{th} impact (subscript before the semicolon). Equations (55), (57) also imply that the orders of approximation of the pre-impact angular velocity at the end of any cycle of rotation (i.e. at $\tau = \tau_{n+1}$) are related with the pertinent orders of approximation of the post-impact angular velocity at the beginning of that cycle of rotation (i.e. at $\tau = \tau_n$):

$$\begin{aligned}\Phi_{n;1}^{(1)} \Big|_{\tau_{n+1}} &= \Phi_{n;1}^{(1)} \Big|_{\tau_n} + 2\Phi_{n;0}^{(2)} \Big|_{\tau_n} T_{n;1} \\ \Phi_{n;2}^{(1)} \Big|_{\tau_{n+1}} &= \Phi_{n;2}^{(1)} \Big|_{\tau_n} + 2\Phi_{n;0}^{(2)} \Big|_{\tau_n} T_{n;2} \\ \Phi_{n;3}^{(1)} \Big|_{\tau_{n+1}} &= \Phi_{n;3}^{(1)} \Big|_{\tau_n} + 2\Phi_{n;0}^{(2)} \Big|_{\tau_n} T_{n;3} \\ &+ 2\Phi_{n;2}^{(2)} \Big|_{\tau_n} T_{n;1} + \frac{1}{2} \Phi_{n;1}^{(1)} \Big|_{\tau_n} (T_{n;1})^2 \\ &+ 3\Phi_{n;1}^{(3)} \Big|_{\tau_n} (T_{n;1})^2 + 4\Phi_{n;0}^{(4)} \Big|_{\tau_n} (T_{n;1})^3\end{aligned}\tag{58}$$

Equation (58) verifies Eq. (35) for $n = 0$. Therefore, Eq. (52) can be expressed in the general form:

$$\begin{aligned}\Phi_n^{(1)} &= \Phi_{n;1}^{(1)}\varepsilon^{\frac{1}{2}} + \Phi_{n;2}^{(1)}\varepsilon^{\frac{2}{2}} + \Phi_{n;3}^{(1)}\varepsilon^{\frac{3}{2}} + O\left(\varepsilon^{\frac{4}{2}}\right) \\ \Phi_n^{(2)} &= \Phi_{n;0}^{(2)} + \Phi_{n;2}^{(2)}\varepsilon^{\frac{2}{2}} + O\left(\varepsilon^{\frac{4}{2}}\right) \\ \Phi_n^{(3)} &= \frac{1}{6}\left(\Phi_{n;1}^{(3)}\varepsilon^{\frac{1}{2}} + \Phi_{n;2}^{(3)}\varepsilon^{\frac{2}{2}} + \Phi_{n;3}^{(3)}\varepsilon^{\frac{3}{2}}\right) \\ &\quad + \Phi_{n;1}^{(3)}\varepsilon^{\frac{1}{2}} + \Phi_{n;3}^{(3)}\varepsilon^{\frac{3}{2}} + O\left(\varepsilon^{\frac{4}{2}}\right) \\ \Phi_n^{(4)} &= \Phi_{n;0}^{(4)} + \Phi_{n;2}^{(4)}\varepsilon^{\frac{2}{2}} + O\left(\varepsilon^{\frac{4}{2}}\right)\end{aligned}\quad (59)$$

where:

$$\begin{aligned}\Phi_{n;0}^{(2)} &= -\frac{i_n + ad_{n;0}}{2}, & \Phi_{n;2}^{(2)} &= -\frac{a}{2}d_{n;2} \\ \Phi_{n;1}^{(3)} &= -\frac{a\omega}{6}c_{n;1}, & \Phi_{n;3}^{(3)} &= -\frac{a\omega}{6}c_{n;3} \\ \Phi_{n;0}^{(4)} &= \frac{-i_n + a(\omega^2 - 1)d_{n;0}}{24}, & \Phi_{n;2}^{(4)} &= \frac{a(\omega^2 - 1)}{24}d_{n;2}\end{aligned}\quad (60)$$

Substituting Eqs (43), (59) into Eq. (44):

$$\begin{aligned}&\left[\Phi_{n;1}^{(1)} + \Phi_{n;0}^{(2)}T_{n;1}\right]\varepsilon^{\frac{1}{2}} + \left[\Phi_{n;2}^{(1)} + \Phi_{n;0}^{(2)}T_{n;2}\right]\varepsilon^{\frac{2}{2}} \\ &+ \left[\begin{array}{l} \Phi_{n;3}^{(1)} + \Phi_{n;0}^{(2)}T_{n;3} + \Phi_{n;2}^{(2)}T_{n;1} \\ + \frac{1}{6}\Phi_{n;1}^{(3)}(T_{n;1})^2 + \Phi_{n;1}^{(3)}(T_{n;1})^2 \\ + \Phi_{n;0}^{(4)}(T_{n;1})^3 \end{array}\right]\varepsilon^{\frac{3}{2}} + O\left(\varepsilon^{\frac{4}{2}}\right) = 0\end{aligned}\quad (61)$$

Equation (61) yields a hierarchy of equations in powers of ε . Observe from Eqs (60), (51) that $\Phi_{n;0}^{(2)}$ and $\Phi_{n;0}^{(4)}$ take different form based on the sign of rotation i_n (e.g. $i_n = i_0$ or $i_n = -i_0$), which, subsequently, governs the form of Eq. (61). Accordingly, there is a need to examine two cases: $i_n = i_0$ and $i_n = -i_0$.

Rotation of same sign as the sign of initial rotation ($i_n = i_0$)

When the sign of rotation i_n of the block after the n^{th} impact is the same as the sign of the initial rotation i_0 (Fig. 2), $\Phi_{n;0}^{(2)} = 0$ (Eq. (60)) and thus solution of Eq. (61) for each order of ε gives:

$$\begin{aligned}\varepsilon^{\frac{1}{2}} : \Phi_{n;1}^{(1)} &= 0 \\ \varepsilon^{\frac{2}{2}} : \Phi_{n;2}^{(1)} &= 0 \\ \varepsilon^{\frac{3}{2}} : \Phi_{n;0}^{(4)}(T_{n;1})^3 + \Phi_{n;1}^{(3)}(T_{n;1})^2 + \Phi_{n;2}^{(2)}T_{n;1} + \Phi_{n;3}^{(1)} &= 0\end{aligned}\quad (62)$$

where $\Phi_{n;0}^{(4)}$, $\Phi_{n;1}^{(3)}$ and $\Phi_{n;2}^{(2)}$ are given by Eq. (60), while $\Phi_{n;3}^{(1)}$ by Eq. (57). The first two equations of Eq. (62)

verify the result of Eq. (27) for $n = 0$. Furthermore, the cubic equation of Eq. (62) has only one real positive root $T_{n;1}$, which represents the lowest order of approximation of the chattering time when $i_n = i_0$.

Rotation of opposite sign to the sign of initial rotation ($i_n = -i_0$)

When the sign of rotation i_n of the block after the n^{th} impact is opposite to the sign of the initial rotation i_0 (Fig. 2), $\Phi_{n;0}^{(2)} \neq 0$ (Eq. (60)) and thus solution of Eq. (61) for each order of ε gives:

$$\begin{aligned}\varepsilon^{\frac{1}{2}} : T_{n;1} &= -\frac{\Phi_{n;1}^{(1)}}{\Phi_{n;0}^{(2)}} \\ \varepsilon^{\frac{2}{2}} : T_{n;2} &= -\frac{\Phi_{n;2}^{(1)}}{\Phi_{n;0}^{(2)}} \\ \varepsilon^{\frac{3}{2}} : T_{n;3} &= -\frac{\left(\Phi_{n;3}^{(1)} + \Phi_{n;2}^{(2)}T_{n;1} + \frac{1}{6}\Phi_{n;1}^{(3)}(T_{n;1})^2\right) + \Phi_{n;1}^{(3)}(T_{n;1})^2 + \Phi_{n;0}^{(4)}(T_{n;1})^3}{\Phi_{n;0}^{(2)}}\end{aligned}\quad (63)$$

where $\Phi_{n;1}^{(1)}$, $\Phi_{n;2}^{(1)}$ and $\Phi_{n;3}^{(1)}$ are given by Eq. (57), while $\Phi_{n;2}^{(2)}$, $\Phi_{n;1}^{(3)}$ and $\Phi_{n;0}^{(4)}$ by Eq. (60). Note that, due to Eqs (62), (60), Eq. (57) returns $\Phi_{n;1}^{(1)} = 0$ and $\Phi_{n;2}^{(1)} = 0$. Therefore, Eq. (63) simplifies:

$$\begin{aligned}\varepsilon^{\frac{1}{2}} : T_{n;1} &= 0 \\ \varepsilon^{\frac{2}{2}} : T_{n;2} &= 0 \\ \varepsilon^{\frac{3}{2}} : T_{n;3} &= -\frac{\Phi_{n;3}^{(1)}}{\Phi_{n;0}^{(2)}}\end{aligned}\quad (64)$$

The third equation of Eq. (64) yields the lowest order of approximation of the chattering time when $i_n = -i_0$.

Chattering time approximation

Assuming that the lowest orders of approximation, namely $T_{n;1}$ given by solution of the cubic equation of Eq. (62) when $i_n = i_0$ and $T_{n;3}$ given by solution of Eq. (64) when $i_n = -i_0$, are sufficient, the approximated chattering time that brings the block to rest can be expressed as:

$$\tau_{ch} = \sum_{n=0}^{\infty} \Delta\tau_n \sim \underbrace{\sum_{n=2k}^{\infty} T_{n;1}\varepsilon^{\frac{1}{2}}}_{i_n=i_0} + \underbrace{\sum_{n=2k+1}^{\infty} T_{n;3}\varepsilon^{\frac{3}{2}}}_{i_n=-i_0}\quad (65)$$

where k takes integer values from zero to (theoretically) infinity, when chattering ends. Note that Eq. (65) consists of two parts. The first part provides the time-intervals between two consecutive impacts when the

sign of rotation is same as the sign of initial rotation ($i_n = i_0$). The second part provides the time-intervals between two consecutive impacts when the sign of rotation is opposite to the sign of initial rotation ($i_n = -i_0$).

Iterative procedure

Equations (62) and (64) form an iterative algorithm that through the coefficients $T_{n;1}$ and $T_{n;3}$, respectively, approximate the chattering time via Eq. (65) that brings the block to rest.

Step 1: During the first cycle of rotation $n = 0$ (Fig. 2(b)), Eq. (31) provides the lowest order of approximation $T_{0;1}$ of the chattering time when the first impact (τ_1) happens.

Step 2: During the second cycle of rotation $n = 1$ and $i_n = -i_0$ (Fig. 2(b)), Eq. (64) provides the lowest order of approximation $T_{1;3}$ of the chattering time when the second impact (τ_2) happens.

Step 3: During the third cycle of rotation $n = 2$ and $i_n = i_0$ (Fig. 2(b)), solution of the cubic equation of Eq. (62) provides the lowest order of approximation $T_{2;1}$ of the chattering time when the third impact (τ_3) happens.

Next steps: Steps 2, 3 form an iterative algorithm that are applied consecutively till the cubic equation of Eq. (62) returns a negative solution, which indicates the end of the chattering motion.

4 Semi-analytical approximation of the chattering time

This section presents a semi-analytical approach to approximate the chattering time of a rocking block. The aim of this section is, thus, twofold: (i) it verifies the accuracy of the asymptotic scheme proposed in Section 3, and (ii) serves as an alternative method of approximation of the chattering time.

Consider the rocking block of Fig. 1 subjected to a low amplitude (i.e. weak) ground motion (Fig. 2). After a sufficiently large, yet arbitrary, number of impacts n , and just before the accumulation point emerges, the time-interval between consecutive impacts becomes orders of magnitude smaller than the period of the ground excitation. Consequently, there exists an n^{th} impact (Fig. 2(c)) after which the ground acceleration appears as a nonzero constant value δ . Under these conditions, the equation of motion of Eq. (4) can be written as:

$$\phi_n''(\tau) - \phi_n(\tau) + \text{sgn}(\phi_n) = -\delta, \quad \tau \leq \tau_{ex} \quad (66)$$

where τ_{ex} is the dimensionless time-instant the ground excitation ends. Without loss of generality, assuming at the time-instant of the n^{th} impact $\tau = \tau_n$ that

$i_n = -i_0 > 0$ (Fig. 2(c)), the initial conditions are $\phi_n(\tau_n) = 0$ and $\phi_n'(\tau_n) = \phi_n'^+ > 0$.

At $\tau = \tau_{n+1}$ (Fig. 2(c)), impact happens, thus solution of Eq. (66) ($\phi_n(\tau_{n+1}) = 0$) yields the time-interval $\Delta\tau_n = \tau_{n+1} - \tau_n$ between two consecutive impacts at τ_n and τ_{n+1} :

$$\Delta\tau_n = \frac{2}{i_n + \delta} \phi_n'^+ \quad (67)$$

where $\phi_n'^+$ is the post-impact angular velocity of the block at the n^{th} impact, and i_n represents the sign of rocking rotation which is equal to $i_n = \pm 1$. Solution of Eq. (66) also gives the pre-impact angular velocity at $\tau = \tau_{n+1}$ (Fig. 2(c)):

$$\phi_{n+1}'^- = -\phi_n'^+ \quad (68)$$

where, because $\phi_n'^+ > 0$ it follows $\phi_{n+1}'^- < 0$. Equation (68) implies that the pre-impact angular velocity at $\tau = \tau_{n+1}$ is equal (in magnitude) with the post-impact angular velocity at $\tau = \tau_n$. Recall that during forced-rocking, this is not generally true (see e.g. Eq. (58)). However, Eq. (68) represents solely the first order approximation of the angular velocity.

For any subsequent impact $j > n$, Eq. (67) allows for a generalization of the time-interval between two consecutive impacts:

$$\Delta\tau_j = \frac{2}{i_j + \delta} \phi_j'^+ \quad (69)$$

with:

$$\begin{aligned} i_{n+1} &= (-1) i_n = -i_n \\ i_{n+2} &= (-1)^2 i_n = i_n \\ i_{n+3} &= (-1)^3 i_n = -i_n \\ &\dots \\ i_j &= (-1)^{j-n} i_n \end{aligned} \quad (70)$$

and based on Eq. (68):

$$\begin{aligned} \phi_{n+1}'^+ &= \eta \phi_{n+1}'^- = (-1) \eta \phi_n'^+ < 0 \\ \phi_{n+2}'^+ &= \eta \phi_{n+2}'^- = (-1) \eta \phi_{n+1}'^+ = (-1)^2 \eta^2 \phi_n'^+ > 0 \\ \phi_{n+3}'^+ &= \eta \phi_{n+3}'^- = (-1) \eta \phi_{n+2}'^+ = (-1)^3 \eta^3 \phi_n'^+ < 0 \\ &\dots \\ \phi_j'^+ &= (-1)^{j-n} \eta^{j-n} \phi_n'^+ \end{aligned} \quad (71)$$

where η is the coefficient of restitution. Therefore, when the $(n+3)^{\text{th}}$ impact occurs (Fig. 2(c)), Eq. (69) yields:

$$\begin{aligned} \Delta\tau_{n+2} &= \tau_{n+3} - \tau_{n+2} \\ &= \frac{2}{i_{n+2} + \delta} \phi_{n+2}'^+ = \eta^2 \frac{2}{i_n + \delta} \phi_n'^+ = \eta^2 \Delta\tau_n \end{aligned} \quad (72)$$

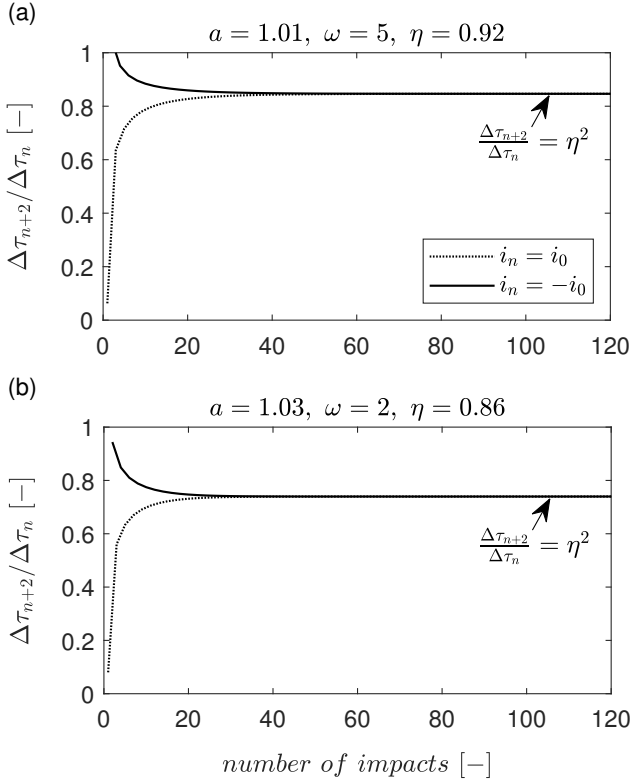


Fig. 3 Ratio of time-intervals of every two consecutive impacts of the rocking block ($p = 1 \text{ s}^{-1}$ and $\alpha = 0.2 \text{ rad}$) of Fig. 1 when subjected to sinusoidal ground excitations of amplitude and frequency (a) $a = 1.01$, $\omega = 5$, (b) $a = 1.03$, $\omega = 2$, respectively, and total duration of T (where $T = 2\pi/\omega$ is the period of the ground excitation) for (a) $\eta = 0.92$ and (b) $\eta = 0.86$, respectively.

Interestingly, Eq. (72) reveals that during the chattering oscillations and after a sufficiently large, yet arbitrary, number of impacts, the ratio of the time-intervals of every two consecutive impacts becomes constant and equal to:

$$\frac{\Delta\tau_{n+2}}{\Delta\tau_n} = \eta^2 \quad (73)$$

Figure 3 plots the ratio of the time-intervals of every two consecutive impacts of the rocking block of Fig. 1 when subjected to sinusoidal ground excitations of different amplitude and frequency. Figure 3 shows that even though the ratio $\Delta\tau_{n+2}/\Delta\tau_n$ initially varies, after a sufficient large number of impacts, it becomes constant. Importantly, Fig. 3 verifies numerically Eq. (73) for different values of the coefficient of restitution η .

From an analytical perspective, Eq. (73) allows for a generalization of the relationship between time-intervals

of every two consecutive impacts.

$$\begin{aligned} \Delta\tau_{n+2} &= \eta^2 \Delta\tau_n \\ \Delta\tau_{n+3} &= \eta^2 \Delta\tau_{n+1} \\ \Delta\tau_{n+4} &= \eta^2 \Delta\tau_{n+2} = \eta^4 \Delta\tau_n \\ \Delta\tau_{n+5} &= \eta^2 \Delta\tau_{n+3} = \eta^4 \Delta\tau_{n+1} \\ &\dots \\ \Delta\tau_j &= \eta^{j-n} \Delta\tau_n \\ \Delta\tau_{j+1} &= \eta^{j-n} \Delta\tau_{n+1} \end{aligned} \quad (74)$$

Therefore, the chattering time can be approximated as a summation of (i) the first sufficiently large, yet arbitrary, n number of impacts for which the ratio of the time-intervals of every two consecutive impacts varies (see e.g. $n < 60$ in Fig. 3) and (ii) the remaining impacts, till chattering is completed, for which the ratio of the time-intervals of every two consecutive impacts becomes constant.

$$\begin{aligned} \tau_{ch} &= \sum_{i=0}^{\infty} \Delta\tau_i = \sum_{i=0}^{n-1} \Delta\tau_i + \sum_{i=n}^{\infty} \Delta\tau_i \\ &\cong \sum_{i=0}^{n-1} \Delta\tau_i + \frac{\Delta\tau_n + \Delta\tau_{n+1}}{1 - \eta^2} \end{aligned} \quad (75)$$

which leaves solely the $\sum_{i=0}^{n-1} \Delta\tau_i$, $\Delta\tau_n$ and $\Delta\tau_{n+1}$ time-intervals of consecutive impacts to be numerically determined, thus, the approach is semi-analytical.

5 Complete chattering behaviour

For practical applications, it is important to determine under which conditions a rocking block sustains *complete chattering* and comes to rest (while the ground excitation is still active). Therefore, with reference to sinusoidal ground excitations strong enough to trigger rocking motion, but weak enough to not cause overturning, this section further examines the influence complete chattering might have on rocking response through numerical simulations.

5.1 Conditions for complete chattering

Consider the rocking block of Fig. 1 subjected to the harmonic sinusoidal ground excitation of Fig. 4. At the time-instant the ground acceleration exceeds the minimum threshold of Eq. (1), rocking initiates (denoted as “1st initiation”). Till the next time-instant the ground acceleration exceeds the minimum threshold (denoted as “2nd” initiation), the block exhibits chattering behaviour due to the weak ground excitation and it might

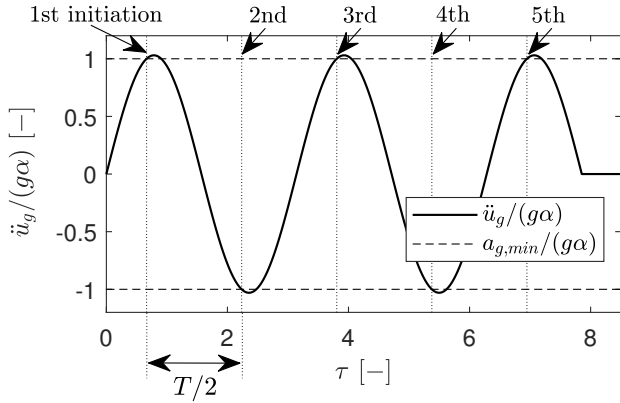


Fig. 4 Harmonic sinusoidal ground excitation of amplitude $a = 1.03$, frequency $\omega = 2$, and total duration of $2.5T$ (where $T = 2\pi/\omega$ is the period of the ground excitation).

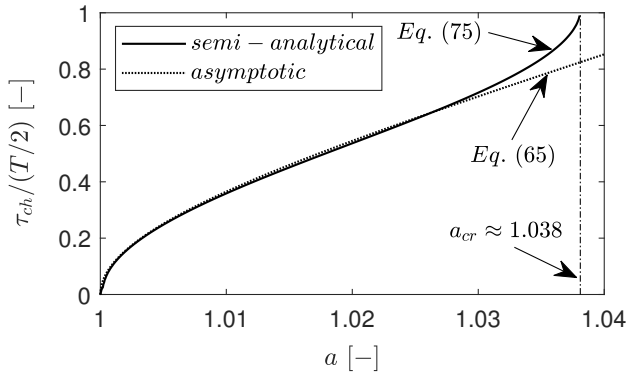


Fig. 5 Semi-analytical (Eq. (75)) and asymptotic (Eq. (65)) approximation of the chattering time τ_{ch} over half the period of the ground excitation $T/2$, where $T = 2\pi/\omega$, with respect to the ground acceleration amplitude a for $\eta = 0.92$.

terminate its motion (or not). In case it terminates its motion, it undergoes complete chattering with its rotation and angular velocity becoming zero even though the ground acceleration remains nonzero. An important realization is that complete chattering can potentially occur only within a maximum time-frame of half the period of the sinusoidal ground excitation, i.e. $\tau_{ch} < T/2$, where τ_{ch} is the (dimensionless) approximated chattering time and $T = 2\pi/\omega$ is the dimensionless period

of the ground excitation. $\tau_{ch} = T/2$ is the maximum potential chattering duration, as after $T/2$ the excitation will again become strong enough to restart rocking even from rest conditions.

Under a sinusoidal ground excitation (Fig. 4), the chattering time τ_{ch} depends on the dimensionless acceleration amplitude a and frequency ω of the ground excitation (Eq. (5)), and the coefficient of restitution η (Eq. (13)). Hence, the ratio of the chattering time over half the period of the ground excitation ($\tau_{ch}/(T/2)$), where $T = 2\pi/\omega$, depends solely on the acceleration amplitude of the excitation a and the coefficient of restitution η . Figure 5 plots the ratio $\tau_{ch}/(T/2)$ with respect to a for $\eta = 0.92$ when the chattering time is approximated through both the proposed asymptotic scheme of Section 3 (Eq. (65)) and the semi-analytical approach of Section 4 (Eq. (75)). Specifically, Fig. 5 reveals the remarkable agreement between the two methodologies and, importantly, verifies the accuracy of the asymptotic approximation. As expected, $\tau_{ch}/(T/2)$ increases with the amplitude of the excitation. Hence, when $\tau_{ch} = T/2$ the value of ground acceleration amplitude is critical, namely a_{cr} in Fig. 5, because the chattering time becomes equal to half the period of the ground excitation, which is its maximum possible value. For higher values of ground acceleration amplitude ($a > a_{cr}$), complete chattering becomes incomplete and the block does not stop rocking. Observe from Fig. 5 that the chattering time presents two asymptotes; one when rocking initiates (i.e. at $a = 1$), and another one when the chattering behaviour transforms from complete to incomplete (i.e. at $a = a_{cr}$). Interestingly, the asymptotic scheme of Section 3 approximates the chattering time almost perfectly both at the asymptote at $a = 1$ and throughout the ground acceleration range, while it fails at the asymptote at $a = a_{cr}$. However, overall, it can be considered that the asymptotic approximation of the chattering time proposed in Section 3 and manifested through Eq. (65) is adequately accurate.

Figure 6 plots the dimensionless critical acceleration a_{cr} with respect to the frequency of the ground excitation ω and the coefficient of restitution η together with a fitted polynomial function (Eq. (76)):

$$a_{cr} = \frac{a_{g,cr}}{g\alpha} = 1.079 - 0.1885\eta + 0.1405\omega + 0.6442\eta^2 - 0.1903\eta\omega - 0.02818\omega^2 - 0.7372\eta^3 + 0.06413\eta^2\omega + 0.03236\eta\omega^2 + 0.00176\omega^3 + 0.2146\eta^4 - 0.0187\eta^3\omega - 0.003045\eta^2\omega^2 - 0.00185\eta\omega^3 \quad (76)$$

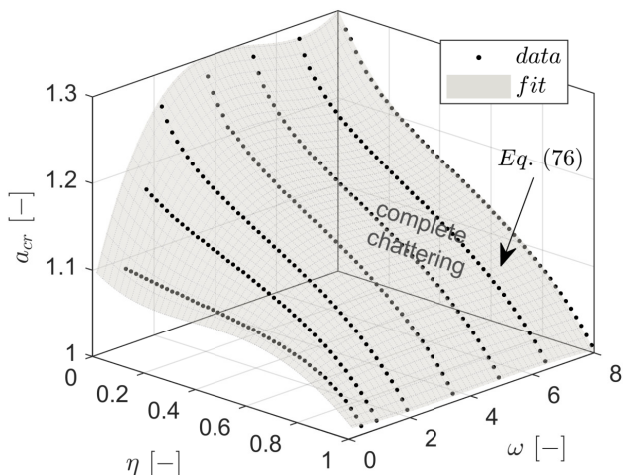


Fig. 6 Critical ground acceleration amplitude a_{cr} , below which complete chattering occurs, with respect to the frequency of the ground excitation ω and the coefficient of restitution η .

In particular, Fig. 6 shows that as η decreases, more energy is dissipated at each impact, thus, a_{cr} increases regardless of the value of ω . Importantly, Fig. 6 illustrates that as η increases, the influence of ω on the critical acceleration a_{cr} decreases. Thus, for values of η within the range of practical interest in rocking dynamics (i.e. $\eta > 0.75$ [24]), a_{cr} can be considered independent of ω .

Figure 7 illustrates such a case, where the critical acceleration a_{cr} is plotted with respect to the frequency of the ground excitation ω for $\eta = 0.92$. Indeed, Fig. 7 verifies the marginal influence of ω on a_{cr} as from $\omega = 0.5$ to $\omega = 8$, a_{cr} differs only by 0.5%. Hence, for such cases, a simplified version of Eq. (76), where a_{cr} depends solely on the coefficient of restitution η , reads:

$$a_{cr} = \frac{a_{g,cr}}{g\alpha} = -0.3941\eta^3 + 0.5763\eta^2 - 0.45\eta + 1.271 \quad (77)$$

Overall, Figs 6, 7 unveil that the critical ground acceleration a_{cr} depends predominately on the coefficient of restitution η —at least for values of η of practical interest. In addition, the value of a_{cr} (Eq. (76) or Eq. (77)) implies that a sinusoidal ground excitation with an acceleration amplitude capable of triggering rocking motion ($a \geq 1$) without overturning, but lower than the critical value ($a < a_{cr}$) leads to complete chattering behaviour. Thus, for $1 < a < a_{cr}$, the block initiates rocking, but approximately after time τ_{ch} , it eventually returns to rest position, without ground motion being zero. If at a subsequent time-instant the ground acceleration exceeds again the minimum thresh-

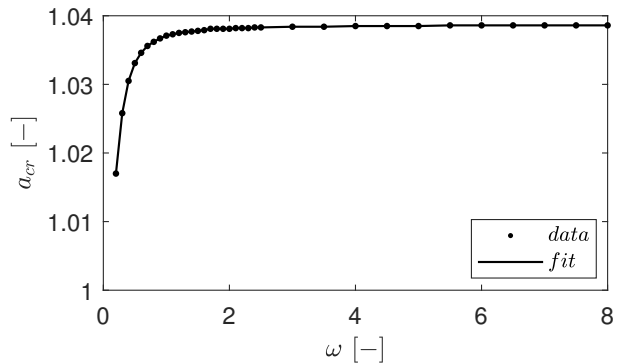


Fig. 7 Critical ground acceleration amplitude a_{cr} with respect to the frequency of the ground excitation ω for $\eta = 0.92$.

old for rocking initiation ($a \geq 1$), the block re-initiates its rocking motion.

5.2 Complete chattering during the dynamic response of a rocking block

This section investigates the effect of complete chattering on the response-history (response-trajectory) of a rocking block. To this end, it performs numerical simulations by means of two time-integration schemes.

The first scheme is a conventional event-based approach which utilises an angular velocity threshold (denoted as “VelTol”) to artificially terminate the chattering oscillations (similar to [9, 10, 27]). According to this approach, if during the integration process the post-impact angular velocity $\phi'_n(\tau_n)$ drops below the assumed threshold, i.e. $|\phi'_n(\tau_n)| \leq \text{VelTol}$, the block terminates its rocking motion and returns to rest position with its rotation and angular velocity being zero. This is one of the simplest and most commonly used methods to avoid instability issues caused by the presence of accumulation points, nevertheless, the specific value of the velocity threshold is arbitrary.

The second scheme is an ad hoc (also event-based) approach, which incorporates the proposed asymptotic approximation of Section 3. Specifically, instead of relying on a velocity threshold, it terminates rocking motion if during the numerical integration process the time-instant τ_n of the n^{th} impact exceeds the approximated chattering time, i.e. $\tau_n \geq \tau_{ch}$. Prerequisite conditions are that the ground excitation is sinusoidal and that its amplitude is lower than the critical ground acceleration amplitude $1 < a < a_{cr}$ (Eq. (76) or Eq. (77)), so complete chattering is expected. The ad hoc scheme re-calculates the chattering time via Eq. (65) if at a subsequent time-instant the ground acceleration exceeds

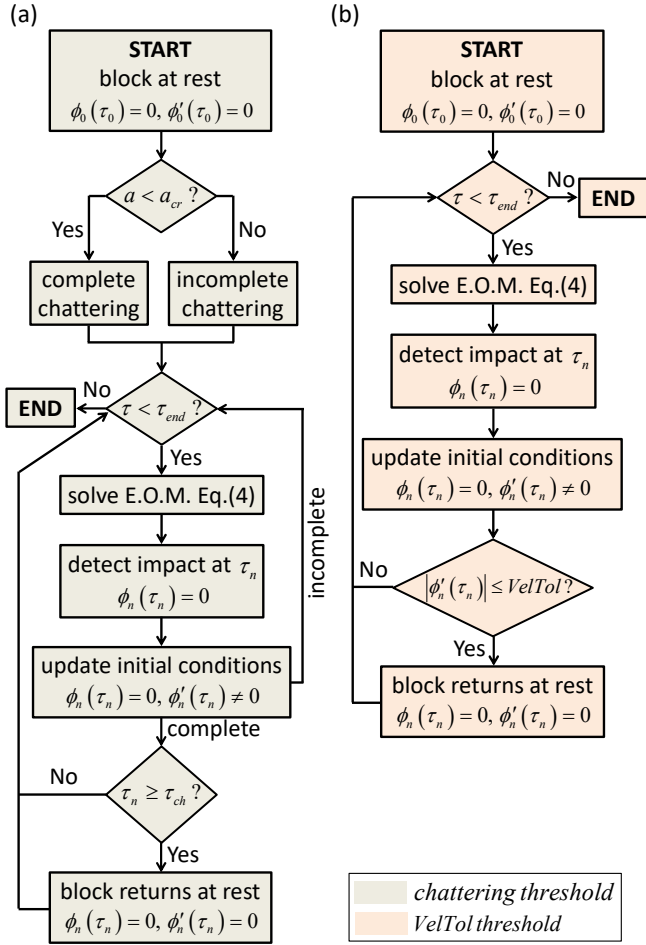


Fig. 8 Flowchart of the examined event-based time-integration schemes of pure rocking response (i.e. no sliding and bouncing effects) of a block without overturning (a) incorporating the asymptotic scheme of the approximated chattering time of Section 3 and (b) utilizing an angular velocity threshold. Note: τ_{end} denotes the total duration of analysis.

again the minimum threshold for rocking initiation ($a \geq 1$). Figure 8 illustrates the flowchart of both numerical schemes. Note that the aim of this section is not to compare time-integration schemes, but to solely investigate the implications of complete chattering on the response-history (response-trajectory) of the rocking block.

Figure 9 plots the response of the rocking block when subjected to a weak sinusoidal ground excitation of amplitude $a = 1.03$ and frequency $\omega = 2$ adopting the ad hoc simulation scheme of Fig. 8(a). For $\eta = 0.92$, the critical ground acceleration amplitude according to Eq. (76) (or Eq. (77)) is $a_{cr} \approx 1.038$ (Fig. 5). Thus, under a lower ground acceleration amplitude (i.e. $a < a_{cr}$), the rocking block is expected to come to rest following its chattering oscillations. Figure 9 shows that, indeed, after rocking commences (denoted as “1st initiation”), the ground acceleration remains low, thus, the block sustains multiple impacts and returns to its ini-

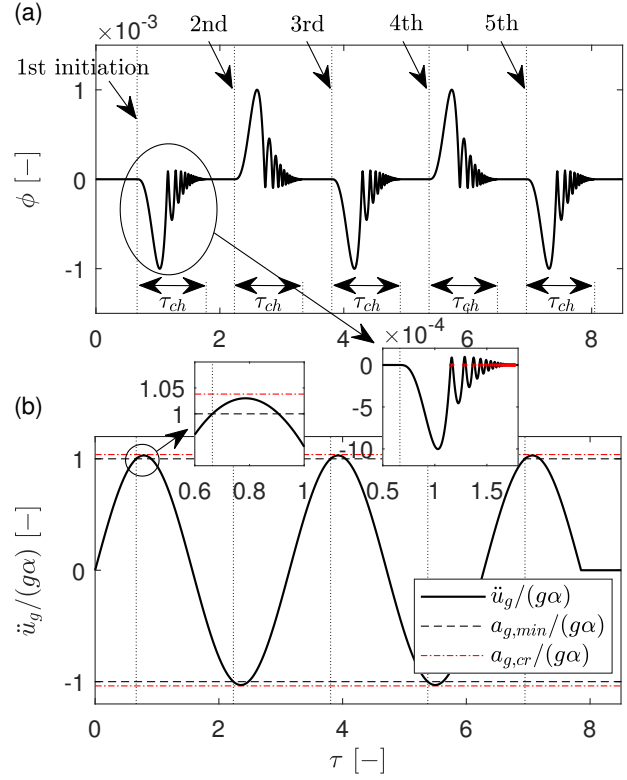


Fig. 9 (a) Response of the rocking block ($p = 1 \text{ s}^{-1}$ and $\alpha = 0.2 \text{ rad}$) of Fig. 1 when subjected to (b) a harmonic sinusoidal ground excitation of amplitude $a = 1.03$, frequency $\omega = 2$, and total duration of $2.5T$ (where $T = 2\pi/\omega$ is the period of the ground excitation) for $\eta = 0.92$.

tial rest position after completing its chattering oscillations, namely approximately after τ_{ch} (Eq. (65)). Later, it re-initiates its motion (denoted as “2nd” initiation) when the next lobe of (opposite sign) ground acceleration triggers rocking motion ($a \geq 1$). Observe from the insert of Fig. 9(a) that the number of impacts the block exhibits before coming to rest, while theoretically infinite, numerically is finite (and it is also experimentally) [8]. Regardless, what is most important is that the block terminates its rocking motion even though it is still subjected to nonzero ground excitation. This fact has not received the attention that deserves in earthquake engineering.

Figures 10, 11 plot the response of the rocking block when subjected to harmonic sinusoidal ground excitations of different (low) amplitude, i.e. $a = 1.03$, $a = 1.05$, respectively, and frequency $\omega = 2$ adopting both simulation approaches, namely “chattering threshold” and “VelTol threshold” (Fig. 8). The main difference between Fig. 10 and Fig. 11 lies in that the ground acceleration amplitude in Fig. 10 is lower than the critical acceleration amplitude $a < a_{cr}$ (Eq. (76) or Eq. (77)),

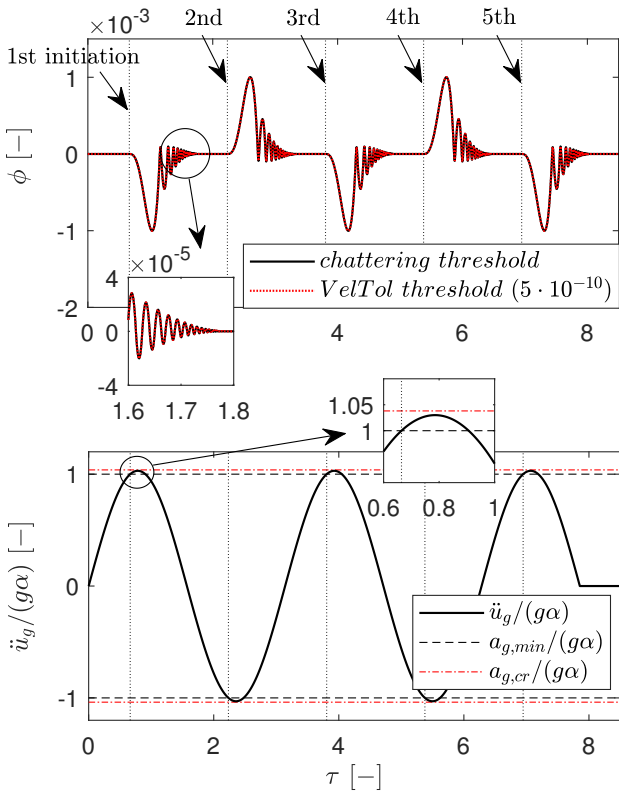


Fig. 10 Response of the rocking block ($p = 1 \text{ s}^{-1}$ and $\alpha = 0.2 \text{ rad}$) of Fig. 1 when subjected to a harmonic sinusoidal ground excitation of amplitude $a = 1.03$, frequency $\omega = 2$, and total duration of $2.5T$ (where $T = 2\pi/\omega$ is the period of the ground excitation) for $\eta = 0.92$. Note: $VelTol/(p\alpha) = 5 \cdot 10^{-10}$ for the “VelTol threshold” simulation.

thus, complete chattering is expected. On the contrary, the amplitude of the ground acceleration in Fig. 11 is higher than the critical value $a > a_{cr}$, hence, incomplete chattering is expected. Recall that for $\eta = 0.92$, it follows $a_{cr} \approx 1.038$ (Eq. (76) or Eq. (77)) (Fig. 5). In addition, Figs 10, 11 reveal the sensitivity of the rocking response on the value of the velocity threshold. As a first approach, Fig. 10 shows that both simulation approaches predict complete chattering (since $a < a_{cr}$) and terminate properly the motion of the block. In particular, after rocking commences (denoted as “1st initiation”), due to the weak, yet nonzero, ground excitation, the block sustains complete chattering and returns to its initial rest position after multiple impacts (see insert of Fig. 10). The block re-initiates its motion (denoted as “2nd” initiation) with zero initial conditions (i.e. rotation and angular velocity) when the next lobe of (opposite sign) ground acceleration is capable of triggering rocking motion ($a \geq 1$). In contrast, in Fig. 11 incomplete chattering is expected since $a > a_{cr}$. In particular, if during the chattering oscillations of the rocking block, its (post-impact) angular velocity drops

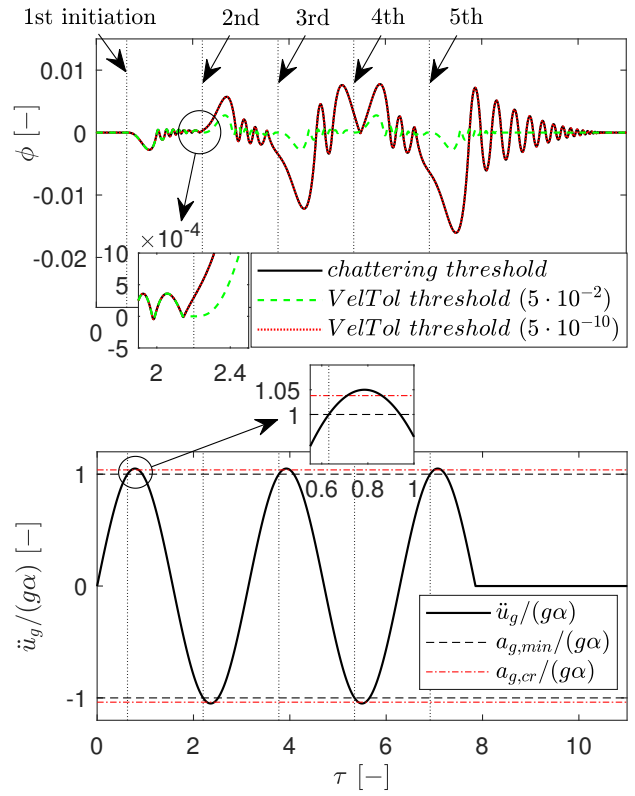


Fig. 11 Response of the rocking block ($p = 1 \text{ s}^{-1}$ and $\alpha = 0.2 \text{ rad}$) of Fig. 1 when subjected to a harmonic sinusoidal ground excitation of amplitude $a = 1.05$, frequency $\omega = 2$, and total duration of $2.5T$ (where $T = 2\pi/\omega$ is the period of the ground excitation) for $\eta = 0.92$. Note: $VelTol/(p\alpha) = 5 \cdot 10^{-2}$ and $VelTol/(p\alpha) = 5 \cdot 10^{-10}$ for the “VelTol threshold” simulations.

below the assumed threshold, the block comes to rest even though it should not have. Subsequently, it enters its next cycle of rotation with zero initial conditions which results in a completely different response-history (response-trajectory). The insert of Fig. 11 clearly illustrates the details of motion termination and subsequently the conditions of rocking re-initiation, when an “improper” value of velocity threshold is adopted. In summary, Figs 10, 11 reveal that the ad hoc “chattering threshold” simulation approach is robust and depends solely on the ground acceleration amplitude criterion of Eq. (76) (or Eq. (77)) that dictates whether chattering is complete or incomplete. The ad hoc simulation approach, though, is applicable only to sinusoidal ground excitations.

Overall, Figs 10, 11, unveil that the chattering phenomenon should not be overlooked when the amplitude of the sinusoidal ground acceleration falls within $1 < a < a_{cr}$. Moreover, in (conventional) event-based simulations, the value of the velocity threshold should be carefully selected. For more complicated waveforms,

i.e. consisted of more lobes of different amplitude or realistic (historic) earthquake records, the influence of complete chattering could be even more complicated and merits further investigation. Currently, no criteria for the appearance of complete chattering, or estimations of its duration, exist for more complicated ground excitation waveforms.

6 Conclusions

The present paper focuses on the *chattering* behaviour of a freestanding rocking block when subjected to a sinusoidal ground excitation of low amplitude. Chattering is a highly nonlinear phenomenon that can have substantial influence on the dynamic response. Specifically, complete chattering refers to the phenomenon during which the block undergoes a theoretically infinite sequence of decaying impacts, in finite time, which eventually bring the block to rest even under nonzero ground excitation. This study investigates the conditions under which complete chattering brings the block to rest, and the corresponding (chattering) time needed for this to happen. To this end, this study adopts perturbation theory and conducts asymptotic analysis.

Specifically, this paper provides an asymptotic scheme that efficiently approximates chattering time, contributing with a state-of-the-art mathematical formulation of the chattering phenomenon for the rocking problem. Note that the proposed asymptotic scheme is also verified through an independent semi-analytical approximation. In addition, this work proposes a ground acceleration amplitude threshold (for sinusoidal ground excitations) below which complete chattering occurs. Importantly, numerical simulations reveal the influence complete chattering might have on the dynamic response of a rocking block. These results suggest that chattering should not be overlooked when the ground acceleration amplitude is barely capable of triggering rocking motion without overturning or when random ground motions replicating earthquakes are considered.

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Data availability Data generated during this study are available from the corresponding author on reasonable request.

Conflict of interest The authors declare that they have no conflict of interest.

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